



## ASSESSMENT OF THE STABILITY OF WELLBORE ROCKS DURING THEIR DYNAMIC LOADING

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### Abstract

The article investigated the deformation processes of the well wall, adopted by the self-plastic model of volcanic rocks, and solved the problem of dynamic instability. A method has been developed to estimate the velocity and amplitude characteristics of the well pressure change that prevents the loss of stability of the mountain rocks (that is, to prevent the collapse and collapse of the well-elastic rock rocks in the wall). On the basis of the theoretical study of the relative deformation of the volume-self-elastic mountain rocks at periodic changes of the additional pressure in the spatial space, the conditions for its stationary and unstable variations are established.

### Keywords:

Borehole rocks;  
Dynamic instability;  
Frequency and amplitude characteristics;  
Volume deformation.

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### Introduction

It is well-known that the need for maintaining hydrocarbon coefficient (>35%) in the development of fields is below the world standards (> 35%), which is closely related to the maintenance and enhancement of the existing well stock. The drilling of additional wells, especially in the aquatic area and the increase in the UVA, requires additional costs for the efficient use of the existing well stock. This problem exacerbates the problem of long-term exploitation of bedding resources, which have been modified as a result of geomechanical processes, and the development of hard-to-extract and large-scale deposits. Since the solution of these problems is not possible with the use of relatively low cost vertical drilling practices, over the past few decades, the use of horizontal wells with large and divergent direction has been widely used in world practice.

### Problem setting

About 80% of investments in the commissioning of new fields, development and delivery of products to the consumer falls on drilling. The funding will also be eliminated as a result of uncertainty in the management of complex and hazardous technological operations related to the increase of the drainage rate and the increase in oil supply through the proper design of the pipeline route. However, there are still many unresolved problems related to drilling wells (geological substantiation,

selection of pipeline route and calculation of well structures), designing of technology and related technical equipment (constructive justification of wells), completion and mastering (development of filter nodes) [1-3].

Theoretical and experimental studies of the tensile state of mountain rocks [4-5] show that these problems are mainly deformation processes of laminated mountain rocks, which are considered as the most suitable model by the self-plastic model.

### Solution of the problem

The loading and discharge stages of the volcanic viscous-plastic mountain rocks can be written according to the following differential equations (4):

- for the loading process

$$m_l \frac{d^2\theta_l}{dt^2} + \eta_l \frac{d\theta_l}{dt} + k_l \theta_l \leq P_l - P_s - \sigma_{fr} \quad (1)$$

- for the unloading process

$$m_{ul} \frac{d^2\theta_{ul}}{dt^2} + \eta_l \frac{d\theta_{ul}}{dt} + k_{ul} \theta_{ul} \leq P_{ul} - P_s - \sigma_{rt} \quad (2)$$

here and there  $\theta_l$  and  $\theta_{ul}$  - respectively, relative dimensional deformations for loading and unloading processes;

$P_l$ ,  $P_{ul}$  - pressures arising from appropriate processes;

$k_l$ ,  $k_{ul}$  - are volume compression modules in these processes;

$P_s$  - side pressure of mountain rocks;

$\sigma_{fr}$  - resistance to hydraulic fracturing of mountain rocks;

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<http://dx.doi.org/10.5510/OGP20200200429>

$\sigma_{rt}$  - resistance of mountain rocks to tension;

$$m_i = \frac{\pi D}{g} P_i, m_{ul} = \frac{\pi D}{g} P_s - \text{are the masses brought}$$

into the unit length of the borehole;

$D$  - Diameter of well;

$g$  - acceleration of gravity;

$\eta, k$  - respectively, the coefficients that characterize the viscosity and elasticity of mountain rocks.

In general, the relative volume deformation of the rocks is defined as the power function of the pressure in the borehole:

$$\theta_l = aP^{b_1} \quad (3)$$

If the loading process is with residual deformation, then

$$\theta_{ul} = \theta_0 + a_1 P_{ul}^{b_1} \quad (4)$$

Thus, the equilibrium equation of viscous-elastic mountain rocks (including some sands, marls, shale etc.) in well conditions can be expressed by the following equation:

$$m\theta''(t) + \eta\theta'(t) + k\theta(t) + P_s - P_{fr} - \Delta P = 0 \quad (5)$$

Equation (5) represents a homogeneous viscous-elastic environment model of mountain rocks and their direct contact with neutral drilling mud. The physical and mechanical interaction of mountain rocks with drilling mud is different and has not been studied until now.

It is well known that the pressure drop in the borehole can be designated as

$$\Delta P = \gamma H - P_{rl} + P_{add} \quad (6)$$

here is:  $\gamma$  - the specific gravity of the drilling mud;

$H$  - depth of well;

$P_{rl}$  - reservoir layer pressure;

$P_{add}$  - an additional pressure that occurs in annular space.

Usually this pressure is assumed to be constant [4]. In fact, the pressure of the drilling mud changes in a annular space while performing various technological processes in the drilling process. Therefore, the following approximate dependencies can be assumed to change this pressure:

$$\Delta P = \gamma H - P_{rl} + P_{add} \quad (7)$$

$$P_{add} = c \cos \omega t$$

where  $c$  and  $\omega$  -the amplitude and frequency of additional pressure changes, respectively.

In general, the pressure changes in a annular space can be represented as the Fourier series:

$$P_{add}(l, t) = \sum_{-\infty}^{+\infty} A_i \exp(in\omega t) \quad (8)$$

$$A_i = \frac{\omega}{2\pi} \int_{-\frac{\pi}{\omega}}^{\frac{\pi}{\omega}} f(t) \exp(in\omega t) dt, \quad (n = 0; \pm 1; \pm 2 \dots) \quad (9)$$

For simplifications, we accept that these variables are subject to the expression (7).

If we take (6) and (7) into account (5)

$$(A + B \cos \omega t) \theta''(t) + \eta \theta'(t) + k \theta(t) = F(t) \quad (10)$$

In the equation (10)

$$A = \frac{\pi D}{g} (\gamma H - P_{rl}),$$

$$B = \frac{\pi D c}{g}, \quad (11)$$

$$F(t) = P_{fr} + \gamma H - P_s - P_{rl} - c \cos \omega t.$$

Thus, it is clear that the dynamic stability of the mountain rocks around the well is represented by a heterogeneous equations with variable and periodic coefficients.

The homogeneous periodic solution of equations (10) with a periodicity of  $2T$  is being sought the following equation:

$$\theta(t) = a_1 \sin \frac{\omega t}{2} + b_1 \cos \frac{\omega t}{2} + \dots \quad (12)$$

Given the equation (12) in the homogeneous equation and zeroing the coefficients  $\sin k\omega t$  and  $\cos k\omega t$  of the same set, we construct the system of algebraic equations below:

$$\begin{aligned} a_1 \left( k - A \frac{\omega^2}{4} + \frac{1}{2} B \frac{\omega^2}{4} \right) - b_1 \eta \frac{\omega}{2} &= 0, \\ b_1 \left( k - A \frac{\omega^2}{4} - \frac{1}{2} B \frac{\omega^2}{4} \right) - a_1 \eta \frac{\omega}{2} &= 0. \end{aligned} \quad (13)$$

The existence of periodic solutions of the homogeneous equation different from zero is conditioned by the fact that the determinant of the conditional system (13) is zero. Using this condition, we obtain the following critical frequencies equation:

$$\begin{vmatrix} k - A \frac{\omega^2}{4} + \frac{1}{2} B \frac{\omega^2}{4} & -\eta \frac{\omega^2}{4} \\ \eta \frac{\omega^2}{4} & k - A \frac{\omega^2}{4} - \frac{1}{2} B \frac{\omega^2}{4} \end{vmatrix} = 0 \quad (14)$$

From the determinant

$$\omega_{1,2} = \sqrt{\frac{4kA - 2\eta \pm 2\sqrt{\eta^4 + 4k^2 B^2 - 4kA\eta^2}}{A^2 - 0.25B^2}} \quad (15)$$

For all of the values within the range defined by the formula (15), the amplitudes obtained from the homogeneous equation solutions are infinitely increasing. The formulas (15) allow to define the amplitude and frequency of the additional pressure changes in the annular space, which causes the volatile state of the wells borehole, that is the contractions, leaks and spikes in the borehole are inevitable in these pressure changes. For absolute elastic rocks, that is, for the case  $\eta=0$  [7]

$$\omega_{1,2} = 2\sqrt{\frac{k(A \pm B)}{A^2 - 0.25B^2}} \quad (16)$$

It means that for absolute elastic rocks, there is also an amplitude and frequency of additional pressure in the annular space, which causes instability.

The following linear algebraic equations system as a result of the homogeneous solution for periodicity of  $T$ :

$$\begin{aligned} k \cdot b_0 - 0 \cdot a_2 - 0.5 \cdot B \cdot \omega^2 \cdot b_2 &= 0, \\ 0 \cdot b_0 + (k - A\omega^2) \cdot a_2 - \eta\omega b_2 &= 0, \\ 0 \cdot b_0 + \eta\omega a_2 + (k - A\omega^2) \cdot b_2 &= 0. \end{aligned} \quad (17)$$

and as a result the solution of its determinant

$$\begin{vmatrix} k & 0 & -0,5B\omega^2 \\ 0 & k - A\omega^2 & -\eta\omega \\ 0 & \eta\omega & k - A\omega^2 \end{vmatrix} = 0 \quad (18)$$

for determining of the boundaries first instability area on changes the frequency the following expression is received:

$$\omega_{3,4} = \frac{1}{A} \sqrt{kA - 0.5\eta^2 \pm \eta \sqrt{0.25\eta^2 - kA}} \quad (19)$$

The analysis conducted was based on models that assumed linear variation of parameters. If the models include  $\xi$  - nonlinear viscosity coefficients and  $\gamma$  - nonlinear elasticity coefficients, instead of the first homogeneous equation may be used the equation with the nonlinear coefficients:

$$(A + B \cos \omega t) \theta'' + \eta \theta' + \xi \theta^1 \theta^2 + k\theta + \gamma \theta^3 = 0 \quad (20)$$

The solution of equation (20) can be searched as  $2T$  or  $T$  periodicity series like (12). For this purpose, at first the non-linear assemblies are divided into Furye series.

For periodic solutions with  $2T$  periodicity

$$\xi \theta^1 \theta^2 + \gamma \theta^3 = \Phi(a_1 b_1) \sin \frac{\omega t}{2} + \psi(a_1 b_1) \cos \frac{\omega t}{2} + \dots, \quad (21)$$

$$\left. \begin{aligned} \Phi(a_1 b_1) &= \frac{\omega}{2\pi} \int_0^{4\pi\omega-1} (\xi \theta^1 \theta^2 + \gamma \theta^3) \sin \frac{\omega t}{2} dt \\ \psi(a_1 b_1) &= \frac{\omega}{2\pi} \int_0^{4\pi\omega-1} (\xi \theta^1 \theta^2 + \eta \theta^3) \cos \frac{\omega t}{2} dt \end{aligned} \right\} \quad (22)$$

For the reports based on (21) and (22) equations

$$\begin{aligned} \xi \theta^1 \theta^2 + \gamma \theta^3 &= \frac{a^2}{4} (3\gamma a_1 + \xi \omega b_1) \sin \frac{\omega t}{2} + \\ &+ \frac{a^2}{4} (3\gamma b_1 + \xi \omega a_1) \cos \frac{\omega t}{2} + \dots, \\ a^2 &= a_1^2 + b_1^2. \end{aligned} \quad (23)$$

Substituting (21) and (22) in to (20) and if we accept the coefficients on  $\sin k\omega t$  and  $\cos k\omega t$  equal to zero, then

$$a_1 \left( k - A \frac{\omega^2}{4} + \frac{1}{2} B \frac{\omega^2}{4} \right) - b_1 \eta \frac{\omega}{4} + \frac{a^2}{4} (\gamma a_1 3 - \xi \omega b_1) = 0, \quad (24)$$

$$b_1 \left( k - A \frac{\omega^2}{4} - \frac{1}{2} B \frac{\omega^2}{4} \right) + a_1 \eta \frac{\omega}{4} + \frac{a^2}{4} (3\gamma b_1 - \xi \omega a_1) = 0.$$

The condition of the existence of solutions of the system (24) that is different from zero is that its determinant is zero, i.e

$$\begin{vmatrix} k - A \frac{\omega^2}{4} + \frac{a^2}{4} 3\gamma + \frac{1}{2} B \frac{\omega^2}{4} & -\eta \frac{\omega}{2} - \frac{a^2}{4} \xi \omega \\ \eta \frac{\omega}{2} + \frac{a^2}{4} \xi \omega & k - A \frac{\omega^2}{4} + \frac{a^2}{4} 3\gamma - \frac{1}{2} B \frac{\omega^2}{4} \end{vmatrix} = 0 \quad (25)$$

From the solution of the determinant, we determine that

$$a = \sqrt{4 \sqrt{\frac{g_1}{4P^2} - \frac{l}{P} - \frac{2g_1}{P}}} \quad (26)$$

where

$$\begin{aligned} g_1 &= 6\gamma k + \eta \xi \omega^2 - 1.5\gamma \omega^2 A, \\ P &= 9\gamma^2 + \gamma^2 \omega^2, \end{aligned} \quad (27)$$

$$l = k^2 - \frac{1}{2} k \omega^2 A + \frac{1}{4} \eta^2 \omega^2 + \frac{1}{16} A^2 \omega^4 - \frac{1}{64} B^2 \omega^4.$$

Equations (25) and (26) determine the amplitude-frequency dependence of the main variables of equation (20).

Unstable solutions of the equation of (20) are characterized by a lower curve of the resonance curve.

Equation of (20) shows that if nonlinear constants are eliminated, then the unsteady solutions are defined by the boundaries of linear theory, i.e., equations (15) and (16).

Let's investigate the coordinated mode of changes near the main resonance. The solving equation of (20) looking for as under mentioned:

$$\theta(t) = a(t) \sin \frac{\omega t}{2} + b(t) \cos \frac{\omega t}{2} + \dots, \quad (28)$$

Here are  $a(t)$ ,  $b(t)$  - «less variable functions».

Considering series of (27) in equation (20), and after intermediate conversions we can construct the following equation system:

$$\begin{aligned} (A - 0.5B)(a'' - \omega b' - 0.25\omega^2 a) + \\ + \eta(a' - 0.5\omega b) + ka + \Phi(a, b) = 0, \\ (A + 0.5B)(b'' + \omega a' - 0.25\omega^2 b) + \\ + \eta(b' + 0.5\omega a) + kb + \Phi(a, b) = 0. \end{aligned} \quad (29)$$

The system of equations (29) can be simplified according to the requirements of the method of «less variable functions». For this reason, if we do not take into account second-order small slogans, then:

$$\begin{aligned} (A - 0.5B)(0.25\omega^2 a + \omega b') &= \Phi(a, b) + ka - 0.5\eta \omega b, \\ (A + 0.5B)(0.25\omega^2 b - \omega a') &= \psi(a, b) + kb + 0.5\eta \omega a. \end{aligned} \quad (30)$$

Equation of (30) shows that the ratio used for the steady state can be obtained from values

$$a'(t) = b'(t) = 0 \quad (31)$$

By excluding non-linear assemblies in system of (30) for the pre - steady state under mentioned values:

$$\begin{aligned} a(t) &= \alpha \exp(ht), \\ b(t) &= \beta \exp(ht). \end{aligned} \quad (32)$$

can be used, where  $\alpha$ ,  $\beta$ ,  $h$  - are constant coefficients.

Given the expression (32) in (30), we have the following system of equations:

$$\begin{aligned} [(A - 0.5B)0.25\omega^2 - k] \alpha + (\omega h + 0.5\eta \omega) \beta = 0, \\ [(A + 0.5B)0.25\omega^2 - k] \beta - (\omega h + 0.5\eta \omega) \alpha = 0. \end{aligned} \quad (33)$$

We can set an index of increase (decrease) given the existence of solutions other than zero like (32):

$$h = \pm \sqrt{0.015625 B^2 \omega^2 - \left(0.25A - \frac{k}{\omega}\right)^2} - 0.5\eta \quad (34)$$

$h$  - positive values correspond to an increase of the steady changes amplitude and the negative values to an its decrease.

As a result, it is worth noting that based on studies [6] to clarify the effect of reflection changes on the deformation of mountain rocks, it has been found that there are three characteristic parts of the

deformation change curves:

Unresolved deformation area, field of deformation and intensity of self-plastic deformation. Obviously, the deformations identified will be typical of a number of mountain rocks in general. Thus, using the results of the above analysis to determine the coefficients  $k$  and  $\eta$  and to determine the deformation profiles that can occur with the rocks, and to prepare proposals for their elimination.

In order to continue the study of mountain rock instability (10) let us determine the relative changes in volume relative deformation based on the solution of equation. Write this equation explicitly [3,6]:

$$(A + B \cos \omega t) \theta''(t) + \eta \theta'(t) + k \theta(t) = d + c \cos \omega t \quad (35)$$

$$d = \sigma_{qr} + \gamma H - P_B - P_{lay} \quad (36)$$

and seek its solution as follows:

- «2T» periodic solutions

$$\theta(t) = \sum_{n=1}^{\infty} \left( x_n \cos \frac{\omega t}{n} + y_n \sin \frac{\omega t}{n} \right) \quad (37)$$

- «T» periodic solutions

$$\theta(t) = \sum_{n=1}^{\infty} (x_n \cos n\omega t + y_n \sin n\omega t) \quad (38)$$

$x_n, y_n$  - are the coefficients determined by the joint solution of equations (35), (37) and (38).

Let us investigate the characteristic case with periodic variations of «T». (38) We find that

$$\theta'(t) = \sum_{n=1}^{\infty} n\omega (y_n \cos n\omega t - x_n \sin n\omega t) \quad (39)$$

$$\theta''(t) = \sum_{n=1}^{\infty} n\omega^2 (x_n \cos n\omega t + y_n \sin n\omega t) \quad (40)$$

Considering (38) - (40) in (35), we create an infinite linear algebraic equation system for  $x_n, y_n$  to sum the coefficients  $\sin k\omega t$  and  $\cos k\omega t$  to zero:

$x_n, y_n;$

$$(k - \omega^2 A)x_1 - \eta \omega y_1 - \omega^2 B \left( 4 \cdot \frac{1}{2} x_2 \right) = c,$$

$$[k - \omega^2 A(4)]x_2 - \eta \omega 2y_2 - \omega^2 B \left( 1 \cdot \frac{1}{2} x_1 + 9 \cdot \frac{1}{2} x_3 \right) = 0,$$

$$[k - \omega^2 A(9)]x_3 - \eta \omega 3y_3 - \omega^2 B \left( 4 \cdot \frac{1}{2} x_2 + 16 \cdot \frac{1}{2} x_4 \right) = 0,$$

.....

$$[k - \omega^2 A(n)]x_n - \eta \omega(n)y_n - \omega^2 B \left[ (n-1)^2 \cdot \frac{1}{2} x_{n-1} + (n+1)^2 \cdot \frac{1}{2} x_{n+1} \right] = 0,$$

$$(k - \omega^2 A)y_1 - \eta \omega x_1 - \omega^2 B \left( 4 \cdot \frac{1}{2} y_2 \right) = 0,$$

$$[k - \omega^2 A(4)]y_2 - \eta \omega x_2 - \omega^2 B \left( 1 \cdot \frac{1}{2} y_1 + 9 \cdot \frac{1}{2} y_3 \right) = 0, \quad (41)$$

$$[k - \omega^2 A(9)]y_3 - \eta \omega x_3 - \omega^2 B \left( 4 \cdot \frac{1}{2} y_2 + 16 \cdot \frac{1}{2} y_4 \right) = 0,$$

.....

$$[k - \omega^2 A(n)^2]y_n - \eta \omega(n)x_n - \omega^2 B \left[ (n-1)^2 \cdot \frac{1}{2} y_{n-1} + (n+1)^2 \cdot \frac{1}{2} y_{n+1} \right] = 0.$$

There is a simple solution of the system of equations (41) for absolute elastics (status  $\eta=0$ ) mountain rocks:

$$x_1 = -\frac{2d}{B\omega^2}, \quad x_2 = \frac{(k - A\omega^2)x_1 - c}{2B\omega^2},$$

$$x_3 = \frac{2(k - 4A\omega^2)}{9B\omega^2}x_2 - \frac{1}{2}x_1, \quad x_4 = \frac{2(k - 9A\omega^2)}{16B\omega^2}x_3 - \frac{4}{16}x_2,$$

$$x_n = \frac{2[k - (n-1)^2 A\omega^2]}{n^2 B\omega^2}x_{n-1} - \left(\frac{n-2}{n}\right)x_{n-2}, \quad n = 5, 6..(42)$$

In the end result (41) defines a reactive approach to solving the system.

It is also necessary that the deformation changes  $n \rightarrow \infty - d a x_n \rightarrow \infty$  are stable. From the last (42) formula to paying for this condition:

$$\left| \frac{2[k - (n-1)^2 A\omega^2]}{n^2 B\omega^2} \right| < 1 \quad (43)$$

anticipation of inequality is necessary and sufficient.

The latter is conditional, and this result can be used to solve technological issues.

Let's test the appropriateness of the mathematical dependence obtained on the following data. Assuming that mountain rocks are also composed of clay shale, for that  $k=3 \cdot 10^3$  MPa;  $\eta=3 \cdot 10^3$  MPa·c; density of drilling mud -  $\rho=1880$  kg/m<sup>3</sup>; well diameter - is 190 mm; perpetration -  $H-P_{rz}=70$  MPa;  $C=2000$  MPa;  $k = 0.1 \cdot 10^6$  MPa;  $d=800$  MPa;  $\omega=6$  c<sup>-1</sup> [8, 9].

For these parameter values, is determined that  $A=4.86$  and  $B=12.16$

From the system of equations (42)

$$x_1 = -0.365; \quad x_2 = -4.330; \quad x_3 = -20.403;$$

$$x_4 = -48.000; \quad x_5 = -55.819; \quad x_6 \dots x_{15} = 4.064;$$

$$x_{25} = 0.883; \quad x_{35} = -0.319.$$

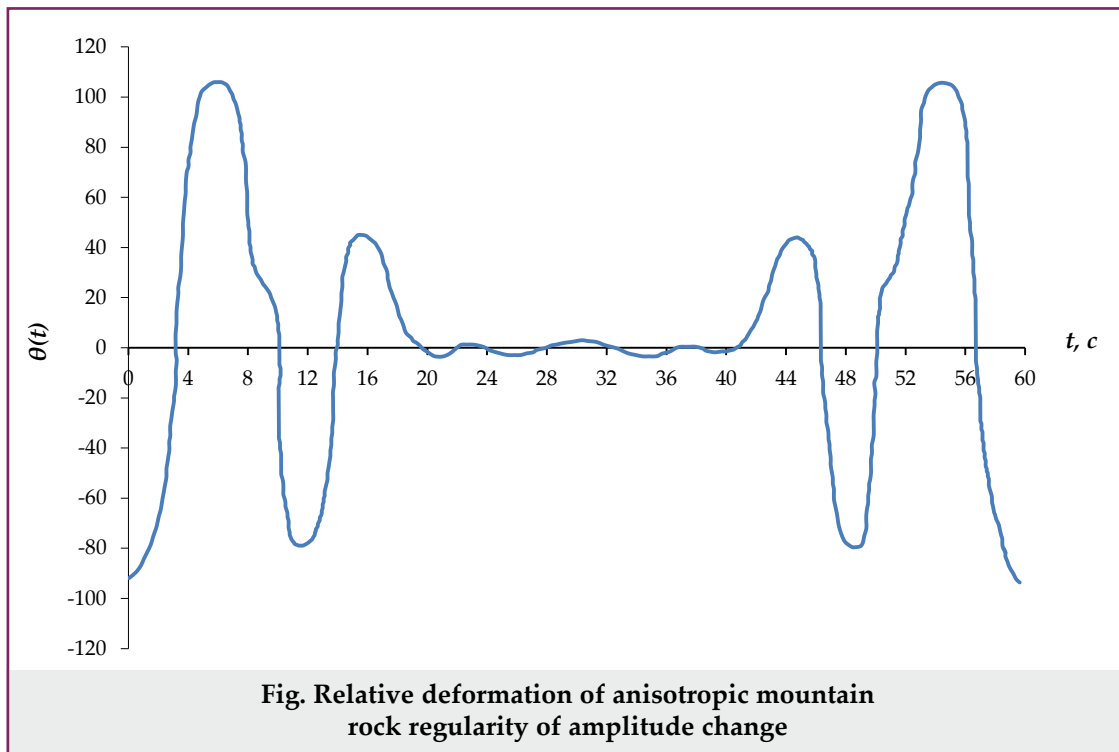
The relative volume rock deformation for its stationary changes (for  $\eta = 0$  condition) can be written according (38) by the below mentioned expression:

$$\theta(t) = \sum_{n=1}^{\infty} x_n \cos n\omega t \quad (44)$$

In table presents the changes the relative volume deformation under the above mentioned conditions.

The obtained results allow estimation of the velocity and amplitude characteristics of the well pressure change, which prevents the process of rock erosion (ie preventing the well wall from collapsing and collapsing on the wall itself).

| Relative volume deformation |          |          |           |           |           |           |           |           |           |           | Table    |
|-----------------------------|----------|----------|-----------|-----------|-----------|-----------|-----------|-----------|-----------|-----------|----------|
| $\omega t$                  | 0        | $\pi/30$ | $2\pi/30$ | $3\pi/30$ | $4\pi/30$ | $5\pi/30$ | $6\pi/30$ | $7\pi/30$ | $8\pi/30$ | $9\pi/30$ | $\pi/3$  |
| $\theta(t)$                 | -124.997 | -102.447 | -83.893   | 11.214    | -0.032    | 0.000     | -0.032    | 11.214    | -83.893   | -102.447  | -124.997 |



### Conclusion

1. Thus, for the first time, the problem of dynamic instability of self-elastic rock rocks has been modeled and solved;
2. The conditions for the presence of stationary and non-stationary changes in the relative deformation of the self-elastic mountain rocks at periodic changes in the additional pressure in the population space;
3. A formula is proposed that allows the determination of the frequency and amplitude characteristics of the increase or decrease of deformation (for elastic rocks).

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## Оценка устойчивости пород при их динамической нагрузке

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### Реферат

В статье рассматриваются деформационные процессы стенки скважины, используется пластическая модель вулканических пород и решается проблема динамической неустойчивости. Разработан метод оценки скоростных и амплитудных характеристик изменений давления в скважине для предотвращения потери устойчивости породы (это предотвращает разрушение стенки упругих пород в скважине). На основе теоретического исследования деформации упругих горных пород, при периодическом изменении дополнительного давления в пространстве, созданы условия для его устойчивости и изменчивости.

**Ключевые слова:** горные породы; динамическая неустойчивость; частотные и амплитудные характеристики; объёмная деформация.

## Dinamik yükləmə zamanı quyu divarı süxurlarının qiymətləndirilməsi

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### Xülasə

Məqalədə quyu divarının deformasiya prosesləri araşdırılmış, vulkanik süxurların plastik modelindən istifadə olunmuş və dinamik qeyri-sabitlik problemi həll edilmişdir. Dağ süxurlarının sabitliyinin itməsinin qarşısını almaq üçün quyu təzyiqinin dəyişməsinin sürətini və amplituda xüsusiyyətlərini qiymətləndirmək məqsədilə bir üsul (metod) hazırlanmışdır (bu, divarda quyunun elastik süxurlarının çökməsinin qarşısını almaq üçündür). Fəzada əlavə təzyiqin dövrü dəyişikliyi zamanı həcmli elastik dağ süxurlarının nisbi deformasiyasının nəzəri tədqiqi əsasında onun sabit və dəyişkən olması üçün şərait yaradılmışdır.

**Açar sözlər:** quyu divarının süxurları; dinamik qeyri-sabitlik; tezlik və amplituda xüsusiyyətləri; həcm deformasiyası.